LARGE-SCALE MODEL STUDY ON CYLINDER GROUPS SUBJECT TO BREAKING AND NONBREAKING WAVES

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Abstract: To investigate group interaction effects on nonbreaking and breaking wave loads on a vertical cylinder as a function of cylinder group configurations and spacing, extensive and systematic large-scale model tests were performed in the Large Wave Channel (GWK) of the Coastal Research Centre (FZK), Hannover. A total of 345 tests, including 15 group configurations with spacing up to three times the cylinder diameter were investigated using regular and irregular nonbreaking wave trains as well as breaking freak waves. The experimental set-up, the test programme and first results of the experiments are reported, showing unexpected interaction effects with regard to the maximum wave loads associated with both breaking and nonbreaking waves.

INTRODUCTION

For the design of marine structures including cylindrical components the influence of neighbouring cylinders on the wave loading (interference, shelter and amplification effects) cannot yet be estimated with sufficient engineering accuracy. Apelt and Piorewicz (1986) reported results of small-scale tests with breaking waves acting on rows of cylinders. They found that breaking wave forces on cylinders in a row in many cases are larger than those on a single isolated cylinder. Smith and Haritos (1997) investigated group interference effects in small-scale experiments. For specific cylinder group configurations Morison-coefficients depending on KCnumber were derived. One of the main problems up to date is the uncertainty associated with the transfer of small-scale model test results to prototype conditions.

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In the Large Wave Channel (GWK) at the Forschungszentrum Küste (FZK), wave loads on Coastal and Offshore structures can be examined in a very large scale, thus minimizing scale effects. The main data of GWK are effective: length 309 m, width 5 m, depth 7 m, maximal water depth 5 m, maximal wave height 2.5 m, breaker height up to 3.2 m. Any kind of waves can be generated: Regular wave trains, solitary waves, wave packets (freak waves) and irregular waves (common and any other spectra measured in the field).

Within the research programme a variety of large-scale experiments with slender cylinders were performed and analysed (Wienke et al., 2000; Wienke, 2001; Sparboom et al., 2001; Irschik et al., 2002; Wienke et al., 2004; Irschik et al., 2004; Oumeraci, 2004). Recently, experiments were carried out with groups of slender cylinders under the attack of nonbreaking and breaking waves.

EXPERIMENTAL MODEL SET-UP

The measuring slender cylinder made of steel with a diameter 0.324 m was installed like a cantilever pile at the support structure. The bending moments of the measuring cylinder were measured by calibrated strain gauge applications. The wave kinematics were recorded synchronously using nine wave gauges and two current meters (Fig. 1). Fig. 2 shows a photo of the model set-up with the measuring cylinder and the support structure.



Fig. 1. Model set-up in the Large Wave Channel (GWK)

Neighbouring cylinders (diameter 0.324 m) were also installed like cantilever piles at the support structure. The spacing between the measuring cylinder and the neighbouring ones with the same diameter were varied up to three times the diameter (3 x D). 15 different configurations of inline, transverse and mixed inline/transverse rows of the piles were investigated with respect to practical applications (Fig. 3). The measuring cylinder is installed in the midline of the wave flume in a distance of 104,69 m from the wave paddle.



Fig. 2. Model set-up with the measuring cylinder (configuration 1, single isolated cylinder)



Fig. 3. Investigated configurations of cylinder groups

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TEST PROGAMME

Regular nonbreaking waves were generated with steepness H/L = 0.0162 - 0.0659, each wave test with 20 waves (Tab. 1). Furthermore, Jonswap wave spectra with at least 100 nonbreaking waves were simulated (Tab. 2).

d = 4.26 m		Wave Period T [s]					
		4	5	6	7	8	
Wave Height H [m]	0.80	×	×	×	×	×	
	1.10	×	×	×	×	×	
	1.40	×	×	×	×	×	

Tab. 1. Test programme for regular nonbreaking waves

1 4	Peak Period T _p [s]			
d = 4.26 m		4	6	8
nt ght	0.80	×	-	-
gnifica ve Hei H _S [m]	1.00	-	×	-
Sig Wa	1.00	-	-	×

Tab. 2. Test programme for Jonswap wave spectra with nonbreaking waves

Breaking freak waves were generated by Gaussian wave packets (Clauss and Kühnlein, 1997; Sparboom et al., 2001; Schmidt-Koppenhagen et al., 2004). The breaking location can be predetermined with accuracy in the simulation procedure for the wave maker control. Different concentration points of the wave packets in front of the cylinder groups and behind the cylinder groups generate different cylinder wave loadings. For this investigations, five different concentration points close to the cylinder groups were selected to get the highest wave loads or bending moments (Tab. 3). It should be noted that the concentration point of a wave packet is not necessarily equal to the breaking point.

Fig. 4 shows a wave test with nonbreaking waves (configuration 7 in Fig. 3) whereas in Fig. 5 a breaking freak wave hits the cylinder group (configuration 12 in Fig. 3).

d = 4.26 m	Location of Concentration Points, Distance from the Wave Paddle [m]				
Wave Height H _{gen} = 1.50 m	103	105	107	109	111
Wave Period $T_{gen} = 6.0 \text{ s}$	×	×	×	×	×

Tab. 3.	Gaussian	wave packets	for the gene	ration of bre	aking freak waves
	0.0000000000000000000000000000000000000				



Fig. 4. Nonbreaking waves (configuration 7)

Fig. 5. Breaking freak wave (configuration 12)

Synchronous time records of the wave height, the horizontal component of the orbital velocity and the inline bending moment are given exemplarily in Figs. 6 - 8. The wave kinematics were recorded with a wave gauge and a current meter at 104.69 m in front of the wave maker (equal to the location of the measuring cylinder). The current meter was installed 1.26 m below SWL which was 4.26 m above bottom (see Fig. 1).

FIRST RESULTS

The maximum measured values of the wave-induced inline bending moments are of special interest for design purposes. It should be noted, that for all cylinder configurations the wave tests were repeated with a very high accuracy. This is an important prerequisite when comparing the wave loads recorded at the measuring cylinder. Configuration 1 was used to measure the wave load (bending moment) of the isolated measuring cylinder. This is the reference wave load to which the measured wave loads of the other configurations 2 - 15 are related in this study.

Wave loads due to regular nonbreaking waves

For different wave periods and all 15 configurations, the maximum inline bending moments are given as a function of the wave height in Fig. 9. Comparing the bending moments, a 60 % increase of the wave load is found for configuration 7 with spacing of 1 x D between three cylinders in a transverse row. For the spacing of 3 x D in configuration 12 a 35 % increase is found whereas in configuration 11 (spacing 3 x D between two cylinders in a transverse row) a 25 % increase is observed. This seems to be in contrast to the common rule of thumb that there is no interaction between two slender cylinders with a spacing larger than 3 x D (CERC, 1984/2003).

It is obvious that no configuration shows a decrease of the wave load compared to the isolated cylinder. Especially, the configurations 2 - 5 show no wave load decrease as could be expected due to shelter effects. On the contrary, an amplification effect up to a 20 % increase for configuration 3, 4 and 5 is found.



Wave loads due to breaking freak waves

Looking to the upper graph in Fig. 10, for the configurations with cylinders in an inline row a decrease of the loading up to 40 % is shown. Wave loading due to wave breaking is influenced by a relatively strong shelter effect.



Fig. 9. Maximum inline bending moments induced by nonbreaking regular waves for the investigated 15 cylinder configurations



Fig. 10. Maximum inline bending moments induced by breaking freak waves with $H_b = 2.60$ m for the investigated 15 cylinder configurations

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For configurations 6 and 7 (1 x D spacing in a transverse row) the wave load increases up to 30 % whereas in the case of a 3 x D gap in configuration 12 the wave load increase is only 10 % (middle graph of Fig. 10). In the mixed configurations 8, 9 and 10 the decrease is much smaller than in configurations 2 - 5. The mixed configurations 13, 14 and 15 show no significant difference of the wave load against the isolated measuring cylinder (lower graph of Fig. 10).

CONCLUDING REMARKS AND OUTLOOK

Since there is yet no reliable formulae available to calculate shelter, interference and amplification effects of closely spaced slender cylinders under wave attack, the results of the large-scale experiments reported in this paper - together with the results of further analysis which is in progress - could be used as a guidance for the design of marine structures with groups of slender cylinders analog to the investigated configurations.

The extensive and systematic experimental programme consists of 345 wave tests with regularly and irregularly generated nonbreaking waves as well as breaking freak waves. The first results on the maximum wave loads reported herein will be extended to a detailed time- and space-dependant analysis of the synchronously measured wave loads and wave kinematics.

The data base can also be used for verification and validation of numerical models (Mittendorf and Zielke, 2004). The "Volume of Fluid (VOF)"-method introduced by Liu (2004) will be applied to develop a design approach for slender cylinders and cylinder groups under wave attack.

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